

Ductal®: from materials to structures

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From concrete nanoscale to structure celebrating Gilles Chanvillard's memory

5-6 July 2016 Champs-sur-Marne, France





Content

- Highlight Gilles extensive and deep contribution to UHPC material development up to his involvement on iconic projects
- Focus on the back analysis method to assess fiber contribution for the post cracking tensile stress
- Key stages of UHPC history from the assessment of fire properties, assessment of structural behavior through pilot projects (crack analysis), shrinkage, etc. up to most recent advanced mix development
- Gilles' footprint on iconic projects as Mucem, Jean Bouin, FLV
- Gilles' human contribution, empowering everybody in the labs combining an amazing teamwork experience with the search for excellence



UHPC

UHPC : Ultra High Performance Concrete

Enhancement of:

- homogeneity
- compactness
- microstructure
- micro-fibre compatibility













Ductal[®]

Ductal[®] = large range of UHPC

- Ductal[®] is a registered trademark owned by LafargeHolcim that covers all the range of UHPC products
- Ductal[®] mixes are patented material customized for specific needs

Performances

- Strong: up to 250 MPa in compressive strength and 50 MPa in flexural strength
- Lifetime of structures is higher than 150 years
- High durability properties
- Constant properties over time







Genesis of UHPC

Reinforced / Prestressed concrete:

compliance of detailings (cover of rebars, spacing between rebars, bending radius of stirrups, ...) leads to large and not optimized sections

Durability: ordinary concrete is sensible to aggressive agent ingress (carbonation, chloride attacks, freeze / thaw cycles) \rightarrow reinforcement must be protected by the concrete





A solution: remove the rebars and develop a high durable and strong concrete





Genesis of Ductal[®]

1996: first extensive project in France (nuclear plant) A total of 2,400 metallic girders were replaced by prestressed girders in Ductal





Poids des sections (kg/ml)





History of Ductal®



Ductal[®]: range of tailor-made mixes



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Vertical





spray, shotcrete







thixotropic

Slope Control





Since ancient times, fibres have been used to reinforce brittle materials

Exodus 5:6,

And Pharaoh commanded the same day the taskmasters of the people, and their officers, saying, we shall no more give the people straw to make brick, as heretofore: let them go and gather straw for themselves

Egyptians used straw to reinforce mud bricks, but there is evidence that asbestos fiber was used to reinforce clay posts about 5000 years ago.









Early 19th century, patent on steel fibers as reinforcement



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Several patents on fibre geometries







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Fibres concrete

- During the early 1960s in the United States, the first major investigation was made to evaluate the potential of steel fibres as reinforcement for concrete. Since then, a substantial amount of research, development, experimentation, and industrial application of steel fibre reinforced concrete has occurred.
- Use of glass fibres in concrete was first attempted in the USSR in the late 1950s. Development of alkali-resistant glass fibres containing zirconia has led to a considerable number of commercialized products (for example architectural cladding panels).
- Initial attempts at using synthetic fibres (nylon, polypropylene) were not as successful as those using glass or steel fibres. However, better understanding of the concepts behind fibre reinforcement, new methods of fabrication, and new types of organic fibres have led researchers to conclude that both synthetic and natural fibres can successfully reinforce concrete.









Fibres concrete: rheological considerations

Whatever their type nor geometry, addition of fibres tends to degrade the rheological behaviour of concrete.



Can we explain such impact with basic considerations ?



Fibres concrete: rheological considerations





Tara Donovan Untitled (Pins)

As for aggregate, it is a question of fibre packing



UHPC: mechanical consideration

Cracking mechanisms of plain concrete: a certain potential of crack bridging provided by aggregates









UHPC: mechanical consideration

Cracking mechanisms of micro fibre concrete: increase of apparent tensile strength but small crack bridging potential





UHPC: mechanical consideration

Pure tension: two main categories, strain softening or hardening



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UHPC : Back analysis method

From pure tension to flexural response:



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Thanks to Gilles, it is now common to characterize the tensile behaviour of fibrereinforced concrete by bending tests rather than by direct tensile tests. Then it is fairly common today to apply reverse analysis to extract the tensile constitutive equation of the material.

For the case of thin plates showing a bending hardening behaviour, Gilles developed an explicit method (which does not need any numerical solver) allowing performing reverse analysis into equivalent strain. This method starts with an experimental moment-deflection relationship to get tensile stress-strain constitutive relationship. The great advantage of this explicit approach is to be easy to program and it allows to quickly manually adjust a constitutive material relationship on the experimental results without any data pre-treatment.



Figure 2: Schematic representation of the non linear distribution of the curvature





Figure 1: Strain and stress distribution over the height of a section

Normal force is obtained by integrating the stress distribution over the section:

$$N_{i} = \int_{S} \sigma.b.dz = \frac{b}{\chi_{i}} \int_{\varepsilon^{i}}^{\varepsilon^{i}} \sigma(\varepsilon).d\varepsilon \quad \text{avec } dz = \frac{d\varepsilon}{\chi_{i}}$$



$$\mathbf{M}_{i+1}^{-} = \frac{\mathbf{b}}{\left(\chi_{i+1}\right)^{2}} \times \left[\int_{\varepsilon^{i+1}}^{\varepsilon^{i}} \sigma(\varepsilon).\varepsilon.d\varepsilon + \int_{\varepsilon^{i}}^{0} \sigma(\varepsilon).\varepsilon.d\varepsilon\right] = \frac{\mathbf{b}}{\left(\chi_{i+1}\right)^{2}} \int_{\varepsilon^{i+1}}^{\varepsilon^{i}} \sigma(\varepsilon).\varepsilon.d\varepsilon + \left(\frac{\chi_{i}}{\chi_{i+1}}\right)^{2} \mathbf{M}_{i}^{-}$$
(12)

The integral term has won a power compared to normal force calculation. The trapezoidal rule method is then no longer acceptable. A variant of Simpson's method is then preferred:

$$\int_{\varepsilon^{i+1}}^{\varepsilon^{i}} \sigma(\varepsilon) \cdot \varepsilon \cdot d\varepsilon = \frac{\sigma(\varepsilon^{i+1}) \cdot \left(2 \cdot \varepsilon^{i+1} + \varepsilon^{i}\right) + \sigma(\varepsilon^{i}) \cdot \left(2 \cdot \varepsilon^{i} + \varepsilon^{i+1}\right)}{6} \times \left(\varepsilon^{i} - \varepsilon^{i+1}\right)$$
(13)

Using (3), compressive stress moment in (11) becomes :

$$M_{i+1}^{+} = \frac{b}{(\chi_{i+1})^{2}} \times \int_{0}^{\varepsilon_{c}^{i}} \sigma(\varepsilon) . \varepsilon . d\varepsilon = \frac{E.b.(\varepsilon_{c}^{i+1})^{3}}{3.(\chi_{i+1})^{2}} = -\frac{E.b}{3.(\chi_{i+1})^{2}} \left[\varepsilon^{i+1}\left(\frac{1-\alpha_{i+1}}{\alpha_{i+1}}\right)\right]^{3}$$
(14)

Finally, with $\chi_{i+1} = -\epsilon^{i+1} \cdot (\alpha_{i+1} \cdot h)^{-1}$, (12), (13) and (14) allow to evaluate the moment at "i+1"





Figure 3: Screen capture of an spreadsheet allowing adjusting the tensile constitutive equation trough the cursors in order to reproduce the experimental results.



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Iconic project: Mucem





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Iconic project: Mucem







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Iconic project: Mucem

- Straight column and Y column were tested
- Modeling and testing were consistent for straight column but unsafe for Y column (larger deflection than expected)







A testing device// failure mode: UHPC or massive head →



Iconic project: Mucem





Iconic project: Mucem



Fibre orientation and distribution

Iconic project: Mucem

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Iconic project: Mucem

Iconic project: Jean-Bouin stadium

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Jean Bouin stadium, France / Architect: Rudy Ricciotti

Iconic project: Jean-Bouin stadium

- Facade: triangle perforated panels, up to 8m x 2.4m
- Roof: waterproof panels + glass inclusions, same dimensions. Ribs devices used for drainage of water
- Isostatic panels support, specific designed hinges

Iconic project: Jean-Bouin stadium

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Iconic project: Jean-Bouin stadium

Ductility and safety factors...

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Iconic project: Jean-Bouin stadium

Iconic project: Fondation Louis Vuitton

Fondation Louis Vuitton (France) / Architect: Frank Gehry

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Iconic project: Fondation Louis Vuitton

- Total area: 9 326 m²
- Panels: 18,737
- Dimensions: 1.5m x 0.4m x 25mm
 - 40% of the panels are flat
 - 60% of the panels are curved and all different

Vacuum system for pouring the curved panels

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Iconic project: Fondation Louis Vuitton

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Iconic project: Fondation Louis Vuitton

Iconic project: Fondation Louis Vuitton

A deep tribute for Gilles Chanvillard

